

PLANNING PROJECT MANAGEMENT ENGINEERING CERTIFICATION





Traffic and Parking Impact Assessment Report

52 Alfred Street, Milson's Point

September 2020

barkerryanstewart.com.au



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Author	AWS / AJ
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SYDNEY Suite 603, Level 6, 12 Century Circuit Norwest Business Park NSW 2153 P (02) 9659 0005 F (02) 9659 0006 E sydney@brs.com.au CENTRAL COAST Studio 5, 78 York Street East Gosford NSW 2250 P (02) 4325 5255 E coast@brs.com.au HUNTER

Unit 1, 17 Babilla Close Beresfield NSW 2322 P (02) 4966 8388 F (02) 4966 1399 E hunter@brs.com.au



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SYDNEY Suite 603, Level 6, 12 Century Circuit Norwest Business Park NSW 2153 **P** (02) 9659 0005 **F** (02) 9659 0006 **E** sydney(@brs.com.au CENTRAL COAST Studio 5, 78 York Street East Gosford NSW 2250 P (02) 4325 5255

E coast@brs.com.au

HUNTER

Unit 1, 17 Babilla Close Beresfield NSW 2322 P (02) 4966 8388 F (02) 4966 1399 E hunter@brs.com.au

List of Abbreviations, Figures and Tables

Abbreviations

DCP	
	Australian Standards, 'AS/NZS 2890.1:2004 Off-Street Car Parking'
AS2890.2	Australian Standards, 'AS 2890.2:2018 Off-Street Commercial Vehicle Facilities'
AS/NZS2890.6	. Australian Standards, 'AS/NZS 2890.6:2002 Off-Street Parking for People with Disabilities'
RMS	Roads and Maritime Services
RMS Guide	

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1 Executive Summary

This Traffic and Parking Impact Assessment has been prepared in accordance with the requirements of the North Sydney Council DCP 2013, the Road and Maritime Services (RMS) 'Guide to Traffic Generating Developments' to accompany a Planning Proposal to North Sydney Council for the development of a mixed-use development.

This Planning Proposal seeks approval for the proposed building height. As such, this report primarily assesses:

- The proposals maximum parking requirement and its ability to appropriately accommodate the parking spaces, and
- The proposals traffic generation and the ability of the surrounding road network to accommodate the traffic generation

The final parking provision is expected to be finalised at the detailed Development Application.

The total proposed provision of 191 spaces, including a covenant for 63 spaces dedicated to Council for the use of the neighbouring commercial developments. The remaining 128 spaces are reserved for the use of the proposed development (residential and commercial / retail uses on-site) which complies with Council DCP requirement of maximum 173 space. It should be noted that the existing car parking structure will be retained for the proposed development. Bicycle parking is also provided in accordance with the minimum requirements set out in the Council DCP.

Taking into account the estimated traffic generation from the proposed development, existing traffic flow conditions and speed environment in Alfred Street South, Glen Street, Dind Street and Fitzroy Street, it is considered that the increase in traffic generated would have a minimal impact on the safety and operating efficiency to these roads, or their intersections. There is no warrant to upgrade these roads or their associated intersections as a result of any additional traffic generated by the development.

The proposed parking and loading facilities have been designed in accordance with the requirements of AS/NZS 2890.1 – Off Street Car Parking, AS 2890.18 – Off-Street Commercial Vehicle Facilities and AS/NZS 2890.6 - Off-street Parking for People with Disabilities. The design of these facilities has sought to preserve the existing structure and layout where possible. They are considered practical and safe ensuring that all traffic generated by the development can enter and exit the site in a forward direction. The estimated required parking can be generally accommodated within the on-site car park.

The Traffic and Parking Impact Assessment concludes that the subject site is suitable for the redevelopment of the site in relation to the impact of traffic, vehicle access, parking and safety considerations. The development is considered to have negligible effect on the safety and operating outcome of the surrounding transport network.

2 Introduction

Barker Ryan Stewart have been engaged by Koichi Takada Architects to prepare a Traffic and Parking Impact Assessment in accordance with the requirements of the North Sydney DCP 2013 (DCP) and LEP 2013 and the Road and Maritime Service's (RMS's) 'Guide to Traffic Generating Developments' to accompany a Planning Proposal for a mixed-use development comprised of commercial / retail tenancies totalling 2,642m² GFA, 159 residential units plus associated basement car parking for 191 vehicles.

A covenant requires 63 parking spaces to be dedicated to Council for the use of the neighbouring development (48-50 and 56 Alfred Street). This covenant would be investigated at the detailed DA phase. The basement layout prepared in the architectural plans show that sufficient parking spaces are available to accommodate these Council spaces and the development parking spaces.

The purpose of this report is to assess and address traffic, access, car parking and pedestrian impacts generated by the proposed development. This can be briefly outlined as follows:

- The expected traffic generation to/from the proposed development.
- The impact of the proposed development on the road network.
- Intersection analysis based on traffic counts.
- Vehicle parking provisions.
- Access design requirements.
- Vehicular requirements for delivery and waste collection.
- Provision for pedestrians.
- Availability of public transport.

3 Existing Conditions

3.1 Site Location

The site is located within a B4 Mixed-Use zone at 52 Alfred Street South, Milsons Point Lot 1/DP738322.

The site is bound by Glen Street to the west, Alfred Street to the east and existing high-rise buildings to the north and south.



Figure 1: Site Location (source: NSW Land & Property Information SIX Maps 2017)

3.2 Existing Development

The site is currently developed and is occupied by an existing commercial tower with basement car parking. Level 1 (ground) features 322.5m² GFA of retail and restaurant space, with levels 2-13 providing a total of 10,205m² commercial GFA. Basement levels B1-B4 currently accommodate 220 parking spaces.



Figure 2: Aerial Photo of Site (source: NSW Land & Property Information SIX Maps 2017)

3.3 Existing Road Conditions

In terms of immediate access to the site, vehicular access to the existing basement is via a vehicular crossing in Glen Street. There is also a ramp connection from the car park to the podium level of 48-50 Alfred Street, which provides access to Alfred Street.

There is full width footpath paving for both street frontages of the site for pedestrians. The site is bound by Glen Street to the west and Alfred Street South to the east, which are both local roads with default speed limits of 50km/h. These roads are two-lane, two-way roads with on-street parking on both sides.

With regards to access to the broader Sydney road network, access to the site from the west and south is restricted by Sydney Harbour due to its location on a peninsula. Therefore, most of the traffic the site will either utilise Fitzroy street to access businesses to the east and the CBD to the south or via Alfred Street South which connects with Lavender Street and the Pacific Highway.

3.4 Traffic Flows and Volumes

Traffic counts were undertaken during the morning and afternoon peak periods to gauge the performance of the current road system.

R.O.A.R Traffic Data undertook traffic counts at the following intersections: Glen Street and Alfred Street South, Alfred Street South and Fitzroy Street, Dind Street and Alfred Street South, site access via Alfred Street South and site access via Glen Street. Counts were undertaken from 7am – 9am and 4pm – 6pm Thursday 7/09/17.

Peak hour periods, traffic volumes and layouts for each of these intersections are summarised below, the full results of the traffic counts are available in Appendix A.



Figure 3: Traffic counts - Alfred Street South/Access AM and PM peak hour



Figure 4: Traffic counts - Dind Street/Alfred Street South AM and PM peak hour



Figure 5: Traffic counts - Alfred Street South/Fitzroy Street AM and PM peak hour



Figure 6: Traffic counts – Glen Street/Alfred Street South AM and PM peak hour

3.5 Public Transport, Pedestrians and Cyclists.

The area is well serviced by public transport, with bus connections and Milson's Point train station located in close proximity to the site. Milson's Point station provides frequent train services to the CBD and Sydney's north on the T1 North Shore Line, which run every 15 minutes during off-peak times and approximately once every 8 minutes during peak times. Given the proposed mixed use residential development replaces an existing commercial premises, it's not anticipated this proposal will have a significant negative impact on peak hour passenger volumes at Milson's Point station. There are also two bus stops located less than 100m north of the site on Alfred Street South service over 20 bus services/routes, with buses stopping every 5 minutes or less during peak hours.

The site is also located within the established Sydney cycle network, close to several shared paths. See Appendix B for more information. Pedestrian infrastructure in the area is also consistent with the high standard generally provided throughout the Sydney CBD.

Overall the existing site has excellent access to pedestrian and cycling infrastructure and public transport, which can accommodate the requirements of the proposed development.

4 Proposed Development

4.1 Development Description

The proposal is to demolish the existing commercial building and construct a mixed-use high-rise building containing 159 residential units and 2,642m² commercial / retail GFA. The unit breakdown is as follows:

- 12 x 1-bedroom units
- 71 x 2-bedroom units
- 76 x 3-bedroom units

The amendments to the existing four-level basement will provide 191 parking spaces, apportioned such that 11 retail parking spaces, 4 commercial spaces and 176 residential spaces are provided. The basement also contains 24 motorcycle spaces and 103 bicycle spaces.

The residential and commercial / retail waste will be stored in separate collection areas and will be serviced by private contractors operating Medium Rigid Vehicle (MRV) type trucks, two MRV loading docks have been provided to facilitate this in accordance with the requirements of Council's DCP.

4.2 Access

The proposed entry/exit to the basement area is located on Glen Street. Residential and commercial / retail car parking will share the same access as will waste collection and loading vehicles.

The use of the basement does not increase the requirements for the design requirements of the existing access driveway with respect to Section 3.2.2 of AS/NZS 2890.1-2004 Parking Facilities – Off Street Car Parking, AS 2890.2-2018 Parking Facilities – Off Street Commercial Vehicle Facilities and Council's DCP requirements.

The entry/exit driveways comply with AS/NZS 2890.1-2004 Parking Facilities – Off Street Car Parking, AS 2890.2-2002 Parking Facilities – Off Street Commercial Vehicle Facilities and Council's DCP requirements.

The proposed driveway locations comply with Figure 3.3 – Minimum Sight Distance for Pedestrian Safety AS/NZS 2890.1 and the proposed driveway gradients comply with AS/NZS 2890.1.

4.3 Service Vehicles

Waste collection will be conducted by a private waste contractor from the collection area near the Glen Street access to the development, accessed via the existing driveway that is to be reconstructed as a part of the development.

It is considered that the shared access of both the waste/loading bay with secondary car park entry/exit will not significantly impact on the flow to and from the overall development as waste collection and deliveries will generally be intermittent and occur outside of peak traffic periods.

Waste and recycling bins will be stored in designated refuse areas. The waste collection contractor will ensure they have the appropriate vehicles to enable collection of waste from with the proposed waste collection area.

5 Car Parking Assessment

5.1 Parking Provision and Requirements

The structure of the car park for the proposed development is largely being retained in its existing configuration with minor amendments to accommodate additional facilities. This acts to minimise demolition and construction and thereby reduce the impact of this development on the local environment. The provision of spaces for the varying uses that will be accommodated on the site are set out in the table below:

Level	Car Retail	Car Commercial	Car Residential	Total Cars	Motor bike
B4	11	4	33	48	1
B3	0	0	47	47	5
B2	0	0	47	47	7
B1	0	0	49	49	5
Total	11	4	176	191	18

Table 1: Car parking Schedule

The proposed car parking and bicycle parking provisions have been assessed against the North Sydney DCP 2013 parking rates contained in table Table B-10.1 for residential uses in a B4 zone outside the St Leonard Precinct and B-10.2 for non-residential uses. A summary of these requirements and the proposed parking provisions are included in tables 2 & 3 below:

Table 2: Car parking requirements and provision

Land Use	North Sydney DCP	Proposed Parking Provision	
Residential Component • 12 x 1-bedroom units • 71 x 2-bedroom units • 76 x 3-bedroom units (Total 159)	<u>Maximum Rates</u> 0.5 spaces per 1-br x 12 = 6 spaces 1 space per 2-br/3-br x 147 = 147 spaces Subtotal = 153 spaces	113 resident car spaces	
Non-Residential Component • 2,642m ² commercial / retail GFA	<u>Maximum Rates</u> 1 space / 400m ² non-residential GFA x 2,642m ² = 7 spaces	15 car spaces	
Other Requirements • Motorcycle Parking	Minimum Rates 1 motorcycle space / 10 car spaces x 160 = 16 spaces	18 motorcycle spaces	
<u>Totals</u>	Max: 160 car spaces Min: 16 motorcycle spaces	191 car spaces – 63 Council spaces = 128 spaces 18 motorcycle spaces (compliant)	

Table 3: Bicycle parking requirements and provision

Land Use	North Sydney DCP	Proposed Parking Provision
 <u>Residential Component</u> 159 units 	Minimum Rates 1 resident space per dwelling x 159 = 159 spaces 1 visitor space per 10 dwellings x 159 = 16 spaces	 104 resident spaces in storage cages B4-B1 55 resident spaces on B4 16 residential visitor spaces on B3
	Subtotal = 175 spaces	Subtotal = 175 (compliant)
Non-Residential Component 2,642m ² commercial / retail GFA (including 968m ² of retail GFA and 1,674 commercial GFA)	Minimum Rates 1 employee space / 150m ² office GFA x 1,674m ² = 12 spaces 1 visitor space / 400m ² office GFA x 1,674m ² = 5 spaces 1 space / 25m ² retail GFA x 968m ² = 39 spaces 2 visitor spaces + (1 / 100m ² retail GFA over 100m ²) = 2 + (868 / 100) = 11 spaces Subtotal = 67 spaces	14 spaces available for commercial employees / visitors and retail visitors on B3 + 60 spaces for retail employees and commercial uses on B4 Subtotal = 74
End of Trip Facilities (Non-Residential) Required Bike Parking	Minimum Rates 1 locker per bicycle space x 67 = 67 lockers 2 shower and change cubicles for up to 20 bikes plus 2 shower and change cubicles per additional 20 bike spaces x 47 = 2.3 (2) shower/change cubicles Subtotal = 67 lockers and 4 shower/change cubicles	51 lockers (no lockers provided for retail or commercial visitor parking) 2 shower / change cubicles More information provided below
Total	247 bicycle spaces	249 bicycle spaces

Proposed on-site motorcycle parking and bicycle parking provisions meet the minimums under the DCP.

End of trip facilities have been provided to service retail employee and commercial employee use, however they have not been provided for customers/visitors to these tenancies. Customers are unlikely to bring ablutions to a restaurant or shop and typically will not be willing to take showers or change clothes. Although there is a deficiency of 2 retail visitor spaces, there is a surplus of bicycle spaces provided for the site as a whole. Nevertheless, it should be noted that the non-compliance regarding the final provision of End of Trip facility is expected to be finalised at the detailed Development Application phase. Overall, it's considered that the above motor vehicle and bicycle parking provisions achieve the development outcomes set out in the North Sydney Council DCP.

5.2 Loading Requirements

The proposed loading facility provision has been assessed against the North Sydney DCP 2013 rates contained in table Table B-10.1 for residential uses in a B4 zone outside the St Leonard Precinct and B-10.2 for non-residential uses. A summary of these requirements and the proposed parking provision is included in the table below:

Table 4: Car parking requirements and provision	
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Land Use	North Sydney DCP	Proposed Parking Provision
 <u>Residential Component</u> 163 dwellings 	<u>Minimum Rates</u> 1 HRV loading bay or 2 MRV loading bays	2 x MRV loading bay (for more information see below)
Non-Residential Component • 2,642m ² commercial / retail GFA	DCP Requirement To be determined by Council	2 x MRV loading bay shared with residential uses
Total Requirements	2 MRV loading bays + commercial requirements from Council	2 x MRV loading bay

Waste collection will be organised by private contractor and may be coordinated with loading requirements for residents and businesses. Building by-laws will also be in place requiring deliveries to the retail and commercial tenancies to take place via vehicles of Medium Rigid Vehicle (MRV) size or lesser.

5.3 Parking Compliance Check

Barker Ryan Stewart has reviewed the plans as provided by Koichi Takada Architects. This review included the layout of car parking and internal roadways / ramps and overall, we are satisfied that the design is consistent with the requirements of Standards AS/NZS 2890.1, AS 2890.2-2002 and AS/NZS 2890.6 and The North Sydney Council DCP. It is anticipated that the car park will function in a satisfactory manner and in accordance with the original design intent. A summary of critical parameters assessed regarding the Australian Standards is included below.

Table 5: Compliance Table

Control	Proposed	Compliance
AS2890.1-2004 (Off-street Car Parking)		
2.4.1 Car Space Dimensions: Class 1 & Class 2	Resident - 5.4m x 2.4m. Min 5.8m aisle width, Retail – 5.4m x 2.5m, min 5.8m aisle	Yes
2.4.2 Blind aisle Extension & Clearance	1m blind aisle extension, min 0.3m clear	Yes
2.4.6 Gradients within parking modules	~0% max cross grade	Yes
2.5.2 Layout Roadways/Ramps	Two-way circulation provided throughout site	See swept paths (Appendix C)
2.5.3 Roadway/Ramp Grades	Max 1:12 (8.3%) entry ramp, max 20.5% internal ramps, length <20m. Transitions of 2m at summit and 3m at base.	Yes
3.2.2 Driveway Width	Category 2, combined 6.5m entry/exit	Yes
3.2.3 Driveway location	Located further than 6m from intersection	Yes
3.3 Gradients of Driveways	<6m at 5%, downgrade to street	Yes
5.2 Column Location/Spacing	Located outside car design envelope	Yes
5.3 Headroom	>2.2m to be provided	Yes
2.4.7 Motorcycle Parking	Motorcycle parking 1.2m x 2.5m	Yes
DCP 10.3.1 P7 Security for residents	Resident parking on upper levels, separated from commercial/retail/council	Yes
A\$2890.2-2002 (Off-street Commercial	Vehicle Facilities)	
3.2.3 Regular Service – Minor Road	(a) Dedicated Service area provided	Yes
	(b) All manoeuvring on-site	
	(c) Forward entry/exit	
	(d) Circulation roadways provided	
3.4.1 Access Driveways	Swept paths show acceptable access and egress for a contractor MRV	Yes
3.3.3 Maximum Grades	Max grade < 15.4%, complying changes of grade may be accommodated.	Yes
3.4.5 Sightlines	Sufficient space available for sufficient sight distance to pedestrians	Yes
4.2 Dimensions of Service Bay-MRV	7m x 9.8m	Yes

Control	Proposed	Compliance
AS2890.6-2009 (Accessible Parking)		
2.2.1 Angle Parking Spaces	2.4m x 5.4m parking and shared spaces + bollard	Yes
2.3 Pavement slope and surface	~Flat 0% grade	Yes
2.4 Headroom	Sufficient space between slabs to provide 2.5m headroom	Yes

6 Traffic Assessment

The impact of the proposed development on the surrounding road network was assessed using SIDRA Intersection modelling software. The traffic counts outlined in section 3.4 and traffic generation estimated below in section 6.1 were used to determine an overall traffic level for the area post-development. Section 6.2 describes how these additional trips were distributed amongst the critical intersections chosen for study. Based on their relatively high volume and critical locations; the intersections of Alfred Street South / Fitzroy Street and Alfred Street South / Glen Street were deemed critical. SIDRA analysis outlined in section 6.3 found that the increased traffic resulting from the proposed development will not have a significant impact on the efficiency of the surrounding road network.

6.1 Traffic Generation

5.1.1 Existing Development

RMS's 'Guide to Traffic Generating Developments', Section 3 – Land Use Traffic Generation (RMS Guide) does not have any available rates for NSW. Instead, the existing traffic generation was calculated using the Institute of Traffic Engineers (ITE) Trip Generation Manual (edition 9) (ITE Guide). The existing estimated traffic volumes generated by the site, described in Section 2.2 are outlined in Table 6 below.

Use	AM peak hour rate	AM peak hour trips	PM peak hour rate	PM peak hour trips
High Density RFB (1 unit)	0.19 / unit	1	0.15 / unit	1
Commercial (10,205m ²)	2/100m ²	205	2/100m ²	205
Restaurant/Retail (322m²)	-	-	5/100m ²	17
	Total AM	206	Total PM	223

Table 6: Existing site use - traffic generation

Note: traffic levels measured at site are significantly lower than the estimations above, potentially due to vacancy of some commercial tenancies in the existing building. For this reason, traffic generation of the proposed development will not be discounted by the estimated traffic of the existing development.

5.1.2 Proposed Development

From the RMS Guide, the proposed estimated traffic volumes generated by the proposed development are outlined in the table below. Residential rates were based on the Guide to Traffic Generating Developments Updated traffic surveys TDT 2013/4A for high density residential developments near train stations.

Table 7: Proposed development - traffic generation

Use	AM peak hour rate	AM peak hour trips	PM peak hour rate	PM peak hour trips
High Density RFB (159 units)	0.19 / unit	30	0.15 / unit	24
Retail Premises (968m ²)	5.6/100m ² -	54	5.6/100m ²	54
Commercial (1,674m ²)	2/100m ²	34	2/100m ²	34
	Total AM	118	Total PM	112

6.2 Trip Distribution

This section outlines the methodology used to distribute the 118 AM and 112 PM trips generated by the proposed development.

- For residential traffic a typical 80-20 outbound/inbound split was adopted for AM traffic and a 20-80 outbound/inbound was chosen for the PM peak.
 - o AM: 25 out / 5 in, PM: 5 out / 19 in
- Commercial traffic was assumed to be 100% inbound during the AM peak and 100% outbound for the PM peak.
 - o AM: 0 out / 34 in, PM: 34 out / 0 in
- Retail traffic during both the AM and PM peaks was assumed to be 80% inbound and 20% outbound. During each peak it was assumed that most customers would head towards the development for either breakfast or dinner time rushes.
 - o AM: 11 out / 43 in, PM: 11 out / 43 in

Total: AM split: 36 out / 82 in, PM split: 50 out / 62 in

Based on the constraints described in Section 3.3 of this report, the following trip distribution estimate has been developed.

- All inbound and outbound trips are distributed equally to north, south, east and west cardinal directions.
 - AM Split to each direction: 9 out / 21 in, PM Split each direction: 12.5 out / 15.5 in.
- Trips OUT to the north and west are assumed to use Glen Street -> Alfred Street South.
 - o AM: 18, PM 25
- Trips OUT to the south and east are assumed to use Glen Street -> Dind Street -> Alfred Street South
 -> Fitzroy Street.
 - o AM: 18, PM 25
- Trips IN from the north and east are assumed to use Fitzroy Street -> Alfred Street South -> Dind Street -> Glen Street.
 - o AM: 42, PM 32
- Trips IN from the south and west are assumed to use Alfred Street South -> Dind Street -> Glen Street.
 AM: 42 PM 32





Figure 7: Site and critical intersections locations (source: NSW Land & Property Information SIX Maps 2017)

Figure 8: Projected traffic volumes at Alfred Street South / Glen Street post-development



Figure 9: Projected traffic volumes at Alfred Street South / Fitzroy Street post-development

6.3 SIDRA Analysis and Impact of Generated Traffic

Intersection performance has been assessed using the SIDRA modelling software which uses the level of service (delay) model adopted by the Roads and Maritime Services (RMS) in NSW to assess intersection performance. Average delay is used to determine the level of service (LOS).

The intersections of Alfred Street South / Glen Street and Alfred Street South / Fitzroy Street were modelled as a network for both the AM and PM peak periods found by the traffic counts in Section 3.4. Traffic volumes modelled for each peak period were based on three scenarios: the existing traffic volume, the predicted traffic volume post-development and the traffic volume for the 'design life' of the intersections. In this case the design life refers to the existing traffic volumes plus volumes generated by the development, compounded annually at 2% growth over 10 years. This growth rate assumption is in keeping with the RMS Guide to Traffic Modelling recommendations.

For each of the three traffic volume scenarios modelled, level of service for all movements through all intersections remains at 'A'. Negligible increases in average delays (on the order of ~1-2 seconds) are anticipated. Considering this there is no warrant to provide any mitigating treatments for the intersections on Alfred Street South. The proposed development will not have a significant impact on the efficiency of the surrounding road network and over its design life of 10 years the existing infrastructure will be suitable to meet the needs of the proposed development. There should be no warrant to upgrade any of the intersections surrounding the development as a result of this development. For more information refer to the detailed SIDRA outputs provided in Appendix D of this report.

7 Conclusion

This Traffic and Parking Impact Assessment has been prepared in accordance with the requirements of the North Sydney DCP 2013, the Road and Maritime Services (RMS) 'Guide to Traffic Generating Developments' to accompany a Planning Proposal to North Sydney Council for the development of a mixed use residential / retail / commercial development.

This Planning Proposal seeks approval for the proposed building height. As such, this report assesses the maximum parking requirement and the appropriateness of the proposal to accommodate these spaces and assess the ability of the surrounding road network to accommodate the development traffic. Accordingly, the parking provision is expected to be finalised at the detailed Development Application stage.

The proposed parking and loading facilities have been designed in accordance with the requirements of AS/NZS 2890.1 – Off Street Car Parking, AS 2890.2 – Off-Street Commercial Vehicle Facilities and AS/NZS 2890.6 - Off-street Parking for People with Disabilities. These facilities are also considered practical and safe ensuring that all traffic generated by the development can enter and exit the site in a forward direction. The estimated required parking is able to be generally accommodated within the on-site car park.

Taking into account the estimated traffic generation from the proposed development, existing traffic flow conditions and speed environment in both Alfred Street South and Glen Street, it is considered that the increase in traffic generated would have a minimal impact on the safety and operating efficiency to these frontage roads, or their intersections with Dind Street and Fitzroy Street.

There would be no warrant to upgrade the local road network as a result of any additional traffic generated by the development.

The total proposed provision of 191 spaces, including a covenant for 63 spaces dedicated to Council for the use of the neighbouring commercial developments. The remaining 128 spaces are reserved for the use of the proposed development (residential and commercial / retail uses on-site) which complies with Council DCP requirement of maximum 160 spaces. The provision of 249 bicycle spaces, 18 motorcycle parking spaces and 2 MRV loading bays also comply with Council DCP requirements.

The Traffic and Parking Impact Assessment concludes that the subject site is suitable for the proposed intensification of use of the site in relation to the impact of traffic, vehicle access, parking and safety considerations. The development is considered to have negligible effect on the safety and operating outcome of the surrounding transport network.

8 References

Australian Standards, 'AS/NZS 2890.1:2004 Off-Street Car Parking'.

Australian Standards, 'AS 2890.2:2002 Off-Street Commercial Vehicle Facilities'.

Australian Standards, 'AS/NZS 2890.6:2002 Off-Street Parking for People with Disabilities'.

Roads and Maritime Services, 'Guide to Traffic Generating Developments' Version 2.2 dated October 2002.

Roads and Maritime Services, 'Guide to Traffic Modelling' Version 1.0 dated February 2013.

Austroads 'Guide to Traffic Engineering Practice, Intersections at Grade – Part 5'.

NSW Department of Planning, 'SEPP (Infrastructure) 2007'.

North Sydney Council's DCP 2013.

Appendix A

Traffic counts



R.O.A.R. DATA

Reliable, Original & Authentic Results

Ph.88196847, Mob.0418-239019

All Vehicles

	NO	RTH	EA	ST	SO	JTH	
	Alfred		St S Fitzroy St		Alfred St S		
Time Per	Ţ	L	<u>R</u>	L	<u>R</u>	<u>I</u>	TOTAL
0700 - 0715	10	96	48	16	11	13	194
0715 - 0730	22	90	37	16	18	10	193
0730 - 0745	21	96	58	11	14	10	210
0745 - 0800	21	120	85	22	20	21	289
0800 - 0815	29	126	67	13	17	23	275
0815 - 0830	36	129	87	17	16	26	311
0830 - 0845	29	95	91	20	17	19	271
0845 - 0900	18	90	77	23	25	23	256
Period End	186	842	550	138	138	145	1999

	NORTH		EAST		SOUTH		
	Alfred St S		Fitzr	Fitzroy St		Alfred St S	
Peak Per	Ī	L	<u>R</u>	L	<u>R</u>	<u>I</u>	TOTAL
0700 - 0800	74	402	228	65	63	54	886
0715 - 0815	93	432	247	62	69	64	967
0730 - 0830	107	471	297	63	67	80	1085
0745 - 0845	115	470	330	72	70	89	1146
0800 - 0900	112	440	322	73	75	91	1113



Client : BarkerRyanStewart

Job No/Name : 6570 MILSONS POINT Traffic Counts

: Thursday 7th September 2017

All Vehicles

Ν

Day/Date

	NO	RTH	EA	ST	SOUTH		
	Alfred St S		Fitzroy St		Alfred St S		
Time Per	Н	L	<u>R</u>		<u>R</u>	Ī	TOTAL
1600 - 1615	17	53	37	9	13	21	150
1615 - 1630	17	81	38	18	5	16	175
1630 - 1645	19	83	32	12	14	22	182
1645 - 1700	16	71	32	14	22	23	178
1700 - 1715	26	100	50	10	17	26	229
1715 - 1730	27	92	38	10	22	27	216
1730 - 1745	27	100	54	18	30	30	259
1745 - 1800	25	97	38	17	29	23	229
Period End	174	677	319	108	152	188	1618

	NORTH		EAST		SOUTH		
	Alfred St S		Fitzroy St		Alfred St S		
Peak Per	Ī	L	<u>R</u>	L	<u>R</u>	I	TOTAL
1600 - 1700	69	288	139	53	54	82	685
1615 - 1715	78	335	152	54	58	87	764
1630 - 1730	88	346	152	46	75	98	805
1645 - 1745	96	363	174	52	91	106	882
1700 - 1800	105	389	180	55	98	106	933







<u>AM</u>

<u>PM</u>





R.O.A.R. DATA

Reliable, Original & Authentic Results Ph.88196847, Mob.0418-239019

All Vehicles

	NO	RTH	WE	ST	SO	JTH	
	Alfred	d St S	Gle	n St	Alfred St S		
Time Per	T	<u>R</u>	L	<u>R</u>	L	<u>T</u>	TOTAL
0700 - 0715	92	0	4	0	0	58	154
0715 - 0730	117	0	8	0	0	46	171
0730 - 0745	134	0	5	0	0	68	207
0745 - 0800	144	0	8	0	0	97	249
0800 - 0815	135	0	11	0	0	81	227
0815 - 0830	171	0	14	0	0	112	297
0830 - 0845	121	0	16	0	0	107	244
0845 - 0900	101	0	8	0	0	87	196
Period End	1015	0	74	0	0	656	1745

		NORTH WEST SOUTH fred St S Glen St Alfred St S					
Peak Per	Ţ	<u>R</u>	L	<u>R</u>	L	<u>T</u>	TOTAL
0700 - 0800	487	0	25	0	0	269	781
0715 - 0815	530	0	32	0	0	292	854
0730 - 0830	584	0	38	0	0	358	980
0745 - 0845	571	0	49	0	0	397	1017
0800 - 0900	528	0	49	0	0	387	964



Client Job No/Name

: BarkerRyanStewart e : 6570 MILSONS POINT Traffic Counts

Db No/Name Day/Date

: Thursday 7th September 2017

All Vehicles

	NO	RTH	WE	EST	SOUTH		
	Alfred St S		Glen St		Alfred St S		
Time Per	T	<u>R</u>	L	<u>R</u>	L	<u>T</u>	TOTAL
1600 - 1615	89	0	4	0	0	66	159
1615 - 1630	96	0	5	0	0	60	161
1630 - 1645	92	0	13	0	0	58	163
1645 - 1700	89	0	14	0	0	57	160
1700 - 1715	115	0	15	0	0	70	200
1715 - 1730	106	0	17	0	0	73	196
1730 - 1745	117	0	13	0	0	84	214
1745 - 1800	114	0	17	0	0	63	194
Period End	818	0	98	0	0	531	1447

	NORTH		WEST		SOUTH		
	Alfred St S		Glen St		Alfred St S		
Peak Per	Ţ	<u>R</u>	L	<u>R</u>	L	<u>T</u>	TOTAL
1600 - 1700	366	0	36	0	0	241	643
1615 - 1715	392	0	47	0	0	245	684
1630 - 1730	402	0	59	0	0	258	719
1645 - 1745	427	0	59	0	0	284	770
1700 - 1800	452	0	62	0	0	290	804







Client : BarkerRyanStewart Job No/Name : 6570 MILSONS POINT Traffic Counts Day/Date : Thursday 7th September 2017

<u>PM</u>



<u>AM</u>



R.O.A.R. DATA

Reliable, Original & Authentic Results

Ph.88196847, Mob.0418-239019

All Vehicles

	NO	RTH	EA	ST	SO	JTH	
	Gle	n St	Acc	ess	Glen St		
Time Per	Ţ	L	<u>R</u>	L	<u>R</u>	Ī	TOTAL
0700 - 0715	1	0	1	0	1	4	7
0715 - 0730	1	0	0	0	2	2	5
0730 - 0745	2	0	0	0	5	7	14
0745 - 0800	2	0	0	0	8	3	13
0800 - 0815	2	0	0	1	1	10	14
0815 - 0830	3	0	0	0	0	17	20
0830 - 0845	2	0	0	0	2	14	18
0845 - 0900	3	0	1	1	6	8	19
Period End	16	0	2	2	25	65	110

	NORTH		EA	ST	SO	JTH	
	Glen St		Access		Glen St		
Peak Per	I	L	<u>R</u>	L	<u>R</u>	I	TOTAL
0700 - 0800	6	0	1	0	16	16	39
0715 - 0815	7	0	0	1	16	22	46
0730 - 0830	9	0	0	1	14	37	61
0745 - 0845	9	0	0	1	11	44	65
0800 - 0900	10	0	1	2	9	49	71



Client : BarkerRyanStewart

: 6570 MILSONS POINT Traffic Counts Job No/Name Day/Date

: Thursday 7th September 2017

All Vehicles

Ν

	NORTH EAST SOUTH						
	Glen St		Access		Glen St		
Time Per	Ī	L	<u>R</u>		<u>R</u>	<u> </u>	TOTAL
1600 - 1615	1	0	1	0	0	15	17
1615 - 1630	3	0	0	1	0	10	14
1630 - 1645	0	0	1	1	0	10	12
1645 - 1700	1	0	3	1	0	17	22
1700 - 1715	2	0	1	4	0	16	23
1715 - 1730	0	0	3	2	0	12	17
1730 - 1745	3	0	2	1	0	15	21
1745 - 1800	3	0	3	0	0	16	22
Period End	13	0	14	10	0	111	148

	NORTH		EA	EAST		JTH	
	Gle	Glen St		Access		n St	
Peak Per	I	<u>L</u>	<u>R</u>	L	<u>R</u>	I	TOTAL
1600 - 1700	5	0	5	3	0	52	65
1615 - 1715	6	0	5	7	0	53	71
1630 - 1730	3	0	8	8	0	55	74
1645 - 1745	6	0	9	8	0	60	83
1700 - 1800	8	0	9	7	0	59	83







<u>AM</u>

: BarkerRyanStewart e : 6570 MILSONS POINT Traffic Counts : Thursday 7th September 2017

<u>PM</u>





R.O.A.R. DATA

Reliable, Original & Authentic Results Ph.88196847, Mob.0418-239019

All Vehicles

	NORTH WE			ST	SO	UTH	
	Alfred	d St S	Acc	ess	Alfred	d St S	
Time Per	<u>T</u>	<u>R</u>	L	<u>R</u>	L	<u>T</u>	TOTAL
0700 - 0715	89	1	0	1	1	55	147
0715 - 0730	117	1	0	0	1	53	172
0730 - 0745	134	0	0	0	1	78	213
0745 - 0800	132	2	0	0	5	102	241
0800 - 0815	160	1	0	0	2	91	254
0815 - 0830	156	4	0	0	1	112	273
0830 - 0845	126	1	0	0	4	111	242
0845 - 0900	91	3	0	0	5	80	179
Period End	1005	13	0	1	20	682	1721

		NORTH Alfred St S			WEST Access		SOUTH Alfred St S	
Р	eak Per	Ţ	<u>R</u>	L	<u>R</u>	L	T	TOTAL
07	700 - 0800	472	4	0	1	8	288	773
07	715 - 0815	543	4	0	0	9	324	880
07	730 - 0830	582	7	0	0	9	383	981
07	745 - 0845	574	8	0	0	12	416	1010
08	300 - 0900	533	9	0	0	12	394	948



Client Job No/Name

: BarkerRyanStewart e : 6570 MILSONS POINT Traffic Counts

Day/Date

e : Thursday 7th September 2017

All Vehicles

	NO	NORTH WEST SOUTH					
	Alfred	Alfred St S		Access		d St S	
Time Per	T	<u>R</u>	L	<u>R</u>	Ŀ	<u>T</u>	TOTAL
1600 - 1615	89	0	1	0	0	68	158
1615 - 1630	94	1	0	2	1	63	161
1630 - 1645	102	0	2	4	0	54	162
1645 - 1700	102	0	2	1	0	52	157
1700 - 1715	119	0	0	1	0	76	196
1715 - 1730	113	0	0	1	0	66	180
1730 - 1745	127	0	1	0	0	91	219
1745 - 1800	124	0	0	3	0	60	187
Period End	870	1	6	12	1	530	1420

	NORTH V		WE	ST	SOUTH		
	Alfred St S		Access		Alfred St S		
Peak Per	Ţ	<u>R</u>	L	<u>R</u>	L	<u>T</u>	TOTAL
1600 - 1700	387	1	5	7	1	237	638
1615 - 1715	417	1	4	8	1	245	676
1630 - 1730	436	0	4	7	0	248	695
1645 - 1745	461	0	3	3	0	285	752
1700 - 1800	483	0	1	5	0	293	782







<u>AM</u>

Client : BarkerRyanStewart Job No/Name : 6570 MILSONS POINT Traffic Counts Day/Date : Thursday 7th September 2017

<u>PM</u>




R.O.A.R. DATA

Reliable, Original & Authentic Results Ph.88196847, Mob.0418-239019

All Vehicles

	NO	RTH	WE	ST	SO	UTH		
	Alfred	Alfred St S		Dind St		Alfred St S		
Time Per	T	<u>R</u>	L	<u>R</u>	Ŀ	<u>T</u>	TOTAL	
0700 - 0715	20	6	6	0	1	18	51	
0715 - 0730	23	16	9	0	0	19	67	
0730 - 0745	6	24	7	0	1	18	56	
0745 - 0800	22	19	12	0	2	28	83	
0800 - 0815	25	19	10	1	1	30	86	
0815 - 0830	22	30	15	1	1	28	97	
0830 - 0845	21	29	8	1	1	29	89	
0845 - 0900	17	31	12	0	2	37	99	
Period End	156	174	79	3	9	207	628	

		NO Alfred	RTH d St S	WE Dine	ST d St	SO Alfred		
	Peak Per	<u>T</u>	<u>R</u>	L	<u>R</u>	L	<u>T</u>	TOTAL
	0700 - 0800	71	65	34	0	4	83	257
	0715 - 0815	76	78	38	1	4	95	292
	0730 - 0830	75	92	44	2	5	104	322
	0745 - 0845	90	97	45	3	5	115	355
	0800 - 0900	85	109	45	3	5	124	371



Client Job No/Name

: BarkerRyanStewart

: 6570 MILSONS POINT Traffic Counts Day/Date

: Thursday 7th September 2017

All Vehicles

	NO	RTH	W	EST	SO	UTH	
	Alfree	d St S	Din	d St	Alfree	d St S	
Time Per	T	<u>R</u>	L	<u>R</u>	L	<u>T</u>	TOTAL
1600 - 1615	10	17	4	0	0	28	59
1615 - 1630	14	22	5	0	1	16	58
1630 - 1645	12	18	11	0	0	25	66
1645 - 1700	11	20	8	1	0	35	75
1700 - 1715	14	21	14	1	0	30	80
1715 - 1730	18	19	15	0	1	37	90
1730 - 1745	22	23	17	0	1	43	106
1745 - 1800	24	20	21	0	1	31	97
Period End	125	160	95	2	4	245	631

	NO	RTH	WE	ST	SO	JTH	
	Alfred St S		Din	d St	Alfred	d St S	
Peak Per	Ţ	<u>R</u>	L	<u>R</u>	L	<u>T</u>	TOTAL
1600 - 1700	47	77	28	1	1	104	258
1615 - 1715	51	81	38	2	1	106	279
1630 - 1730	55	78	48	2	1	127	311
1645 - 1745	65	83	54	2	2	145	351
1700 - 1800	78	83	67	1	3	141	373







Client : BarkerRyanStewart Job No/Name : 6570 MILSONS POINT Traffic Counts Day/Date : Thursday 7th September 2017

<u>PM</u>



<u>AM</u>

Appendix B

Public Transport and Cycling Information

Excerpt of 2012 Sydney Cycle Map



https://www.northsydney.nsw.gov.au/Recreation_Facilities/Walking_Cycling/Cycling/Cycling

Appendix C

Swept Paths





B99 Vehicle (Realistic min radius) (2004) EGRE Overall Length Overall Width Overall Body Height Min Body Ground Clearance Track Width Lock-to-lock time	1.878m 0.272m 1.840m 4.00s
Curb to Curb Turning Radius	6.250m





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mail@brs.com.au

INING - PROJECT MANAGEMENT - ENGINEERING - CERTIFICATION

E 21/02/19

CLIENT AMENDMENTS

WS	Scales: Plan	1:125	Plan No.	
WS	Horiz. Vert.		SY170197SP	1E
ĒA	X-Sec	t.	File Ref.	REV.
			SY170197SP1E.DWG	
	Datum:		SHEET 2 OF 5 SHEETS	



AWS	Scales: Plan	1:125	Plan No.	
AWS	Horiz. Vert.		SY170197SF	21E
ΞA	X-Sect.		File Ref.	REV.
			SY170197SP1E.DWG	
	Datum:		SHEET 3 OF 5 SHEETS	





52 ALFRED STREET, MILSONS POINT - SWEPT PATHS



B99 Vehicle (Realistic min radius) (2004) INGR Overall Length Overall Width Overall Body Height Min Body Ground Clearance Track Width	ESS 5.200m 1.940m 1.878m 0.272m 1.840m
Min Body Ground Clearance	
Track Width	•
Lock-to-lock time	4.00s
Curb to Curb Turning Radius	6.250m



B85 Vehicle (Realistic min radius) (2004) EGRESS
Overall Length4.910m
1.870mOverall Width1.870m
1.421mOverall Body Height1.421m
0.159mMin Body Ground Clearance0.159m
1.770m
4.00sTrack Width1.770m
4.00sLock-to-lock time4.00s
5.750m





OTAL PROJECT SOLUTIONS

NG - PROJECT MANAGEMENT - ENGINEERING - CERTIFICATION

CLIENT AMENDMENTS

E 21/02/19

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			L										
X 3				() () () ()		X 9		TYPICAL CAR SPACE 2400 x 5400					S2
			6000				B4 UPP RL 16.5 ∳						
16		17 6							20 TYPICAL CAR SPACE 2400 x 5400			TO B3 5500	
/ 23	24	25	26	27	сомм. 28	сомм. 29	сомм. 30	сомм. 31	TYPICAL RETAIL CAR SPACI 2600 x 540 RETAIL	RETAIL LIFT	RETAIL / 33	RETAIL	
			6000				B4 LOW RL 15.8 -∲-	/ER 80				1	1000
36 37 57	37 RES ST	38 STORAG (7)	39	40	A1 RETAIL				45	46		RETAIL	

52 ALFRED STREET, MILSONS POINT - SWEPT PATHS

Designed:AWSDrawn:AWSChecked:EA

B85 VEHICLE PARKING AND UNPARKING



B85 Vehicle (Realistic min radius) (2004) INGRESS
Overall Length4.910m
1.870mOverall Width1.870m
1.421mOverall Body Height1.421m
0.159mMin Body Ground Clearance0.159m
1.770m
4.00sTrack Width1.770m
4.00sLock-to-lock time4.00s
5.750m



B85 Vehicle (Realistic min radius) (2004) EGRESS
Overall Length4.910m
1.870mOverall Width1.870m
1.421mOverall Body Height1.421m
0.159mMin Body Ground Clearance0.159m
1.770m
4.00sTrack Width1.770m
4.00sCurb to Curb Turning Radius5.750m



Appendix D

SIDRA Output

Site: 102 [GlenXAlfred - AM - Post Dev]

New Site Giveway / Yield (Two-Way)

Move	ement l	Performa	nce - \	/ehicle	s								
Mov ID	OD Mov	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	South: RoadName												
2	T1	418	5.0	418	5.0	0.221	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Appro	ach	418	5.0	418	5.0	0.221	0.0	NA	0.0	0.0	0.00	0.00	60.0
North	RoadN	lame											
8	T1	513	5.0	513	5.0	0.271	0.0	LOS A	0.0	0.0	0.00	0.00	59.9
Appro	ach	513	5.0	513	5.0	0.271	0.0	NA	0.0	0.0	0.00	0.00	59.9
West:	RoadN	ame											
10	L2	68	0.0	68	0.0	0.063	7.1	LOS A	0.2	1.7	0.44	0.65	48.2
Appro	ach	68	0.0	68	0.0	0.063	7.1	LOS A	0.2	1.7	0.44	0.65	48.2
All Ve	hicles	999	4.7	999	4.7	0.271	0.5	NA	0.2	1.7	0.03	0.04	58.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.0 % Number of Iterations: 5 (maximum specified: 10)

Number of iterations. 5 (maximum specifica. 10)

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V Site: 101 [FitzroyXAlfred - AM - Post Dev]

New Site Roundabout

Move	Movement Performance - Vehicles												
Mov ID	OD Mov	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: RoadN	lame											
2	T1	94	0.0	94	0.0	0.179	4.0	LOS A	1.0	7.2	0.53	0.66	23.6
3	R2	91	0.0	91	0.0	0.179	7.9	LOS A	1.0	7.2	0.53	0.66	32.5
Appro	ach	184	0.0	184	0.0	0.179	5.9	LOS A	1.0	7.2	0.53	0.66	29.3
East:	RoadNa	ame											
4	L2	113	0.0	113	0.0	0.369	4.8	LOS A	2.5	18.3	0.42	0.62	29.5
6	R2	347	5.0	347	5.0	0.369	9.7	LOS A	2.5	18.3	0.42	0.62	28.8
Appro	ach	460	3.8	460	3.8	0.369	8.5	LOS A	2.5	18.3	0.42	0.62	29.0
North	: RoadN	lame											
7	L2	495	5.0	495	5.0	0.472	4.5	LOS A	4.0	28.7	0.38	0.48	43.8
8	T1	158	0.0	158	0.0	0.472	4.7	LOS A	4.0	28.7	0.38	0.48	40.6
Appro	ach	653	3.8	653	3.8	0.472	4.6	LOS A	4.0	28.7	0.38	0.48	43.2
All Ve	hicles	1297	3.2	1297	3.2	0.472	6.2	LOS A	4.0	28.7	0.42	0.55	37.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.0 % Number of Iterations: 5 (maximum specified: 10)

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V Site: 102 [GlenXAlfred - PM - Post Dev]

New Site Giveway / Yield (Two-Way)

Mov	Movement Performance - Vehicles Mov OD Demand Flows Arrival Flows Deg. Average Level of 95% Back of Queue Prop. Effective Average													
Mov ID	OD Mov	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance	Prop. Queued		Average Speed	
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h	
South	n: RoadN	lame												
2	T1	305	5.0	305	5.0	0.162	0.0	LOS A	0.0	0.0	0.00	0.00	60.0	
Appro	oach	305	5.0	305	5.0	0.162	0.0	NA	0.0	0.0	0.00	0.00	60.0	
North	h: RoadN	lame												
8	T1	624	5.0	624	5.0	0.331	0.0	LOS A	0.0	0.0	0.00	0.00	59.9	
Appro	oach	624	5.0	624	5.0	0.331	0.0	NA	0.0	0.0	0.00	0.00	59.9	
West	: RoadN	ame												
10	L2	92	0.0	92	0.0	0.074	6.6	LOS A	0.3	2.1	0.37	0.61	48.5	
Appro	oach	92	0.0	92	0.0	0.074	6.6	LOS A	0.3	2.1	0.37	0.61	48.5	
All Ve	ehicles	1021	4.6	1021	4.6	0.331	0.6	NA	0.3	2.1	0.03	0.05	58.0	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.0 % Number of Iterations: 5 (maximum specified: 10)

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V Site: 101 [FitzroyXAlfred - PM - Post Dev]

New Site Roundabout

Move	Movement Performance - Vehicles Mov OD Demand Flows Arrival Flows Deg. Average Level of 95% Back of Queue Prop. Effective Average													
Mov ID	OD Mov	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued		verage Speed	
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h	
South	: Road	Vame												
2	T1	112	0.0	112	0.0	0.204	3.2	LOS A	1.2	8.2	0.40	0.60	25.1	
3	R2	129	0.0	129	0.0	0.204	7.1	LOS A	1.2	8.2	0.40	0.60	33.5	
Appro	ach	241	0.0	241	0.0	0.204	5.3	LOS A	1.2	8.2	0.40	0.60	30.9	
East:	RoadNa	ame												
4	L2	81	0.0	81	0.0	0.217	4.5	LOS A	1.3	9.3	0.34	0.60	30.2	
6	R2	189	5.0	189	5.0	0.217	9.4	LOS A	1.3	9.3	0.34	0.60	29.7	
Appro	ach	271	3.5	271	3.5	0.217	8.0	LOS A	1.3	9.3	0.34	0.60	29.9	
North:	RoadN	lame												
7	L2	409	5.0	409	5.0	0.418	4.8	LOS A	3.1	22.3	0.41	0.50	43.5	
8	T1	134	0.0	134	0.0	0.418	4.9	LOS A	3.1	22.3	0.41	0.50	40.3	
Appro	ach	543	3.8	543	3.8	0.418	4.8	LOS A	3.1	22.3	0.41	0.50	42.9	
All Ve	hicles	1055	2.8	1055	2.8	0.418	5.7	LOS A	3.1	22.3	0.39	0.55	38.0	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.0 % Number of Iterations: 5 (maximum specified: 10)

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Site: 102 [GlenXAlfred - AM - Design]

New Site Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles Mov OD Demand Flows Arrival Flows Deg. Average Level of 95% Back of Queue Prop. Effective Average													
Mov ID	OD Mov	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued		Average Speed	
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h	
South	: RoadN	lame												
2	T1	418	5.0	418	5.0	0.221	0.0	LOS A	0.0	0.0	0.00	0.00	60.0	
Appro	ach	418	5.0	418	5.0	0.221	0.0	NA	0.0	0.0	0.00	0.00	60.0	
North:	RoadN	lame												
8	T1	513	5.0	513	5.0	0.271	0.0	LOS A	0.0	0.0	0.00	0.00	59.9	
Appro	ach	513	5.0	513	5.0	0.271	0.0	NA	0.0	0.0	0.00	0.00	59.9	
West:	RoadN	ame												
10	L2	68	0.0	68	0.0	0.063	7.1	LOS A	0.2	1.7	0.44	0.65	48.2	
Appro	ach	68	0.0	68	0.0	0.063	7.1	LOS A	0.2	1.7	0.44	0.65	48.2	
All Ve	hicles	999	4.7	999	4.7	0.271	0.5	NA	0.2	1.7	0.03	0.04	58.5	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.0 % Number of Iterations: 5 (maximum specified: 10)

Number of iterations: 5 (maximum specified: 10)

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Site: 101 [FitzroyXAlfred - AM - Design]

New Site Roundabout

Move	Movement Performance - Vehicles													
Mov ID	OD Mov	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Stop Rate	verage Speed	
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h	
South	: RoadN	lame												
2	T1	94	0.0	94	0.0	0.179	4.0	LOS A	1.0	7.2	0.53	0.66	23.6	
3	R2	91	0.0	91	0.0	0.179	7.9	LOS A	1.0	7.2	0.53	0.66	32.5	
Appro	ach	184	0.0	184	0.0	0.179	5.9	LOS A	1.0	7.2	0.53	0.66	29.3	
East:	RoadNa	ame												
4	L2	113	0.0	113	0.0	0.369	4.8	LOS A	2.5	18.3	0.42	0.62	29.5	
6	R2	347	5.0	347	5.0	0.369	9.7	LOS A	2.5	18.3	0.42	0.62	28.8	
Appro	ach	460	3.8	460	3.8	0.369	8.5	LOS A	2.5	18.3	0.42	0.62	29.0	
North	RoadN	lame												
7	L2	495	5.0	495	5.0	0.472	4.5	LOS A	4.0	28.7	0.38	0.48	43.8	
8	T1	158	0.0	158	0.0	0.472	4.7	LOS A	4.0	28.7	0.38	0.48	40.6	
Appro	ach	653	3.8	653	3.8	0.472	4.6	LOS A	4.0	28.7	0.38	0.48	43.2	
All Ve	hicles	1297	3.2	1297	3.2	0.472	6.2	LOS A	4.0	28.7	0.42	0.55	37.3	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.0 % Number of Iterations: 5 (maximum specified: 10)

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V Site: 102 [GlenXAlfred - PM - Design]

New Site Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles Mov OD Demand Flows Arrival Flows Deg. Average Level of 95% Back of Queue Prop. Effective Average													
Mov ID	OD Mov	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance	Prop. Queued		Average Speed	
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h	
South	: RoadN	lame												
2	T1	305	5.0	305	5.0	0.162	0.0	LOS A	0.0	0.0	0.00	0.00	60.0	
Appro	ach	305	5.0	305	5.0	0.162	0.0	NA	0.0	0.0	0.00	0.00	60.0	
North	RoadN	ame												
8	T1	624	5.0	624	5.0	0.331	0.0	LOS A	0.0	0.0	0.00	0.00	59.9	
Appro	ach	624	5.0	624	5.0	0.331	0.0	NA	0.0	0.0	0.00	0.00	59.9	
West:	RoadN	ame												
10	L2	92	0.0	92	0.0	0.074	6.6	LOS A	0.3	2.1	0.37	0.61	48.5	
Appro	ach	92	0.0	92	0.0	0.074	6.6	LOS A	0.3	2.1	0.37	0.61	48.5	
All Ve	hicles	1021	4.6	1021	4.6	0.331	0.6	NA	0.3	2.1	0.03	0.05	58.0	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.0 % Number of Iterations: 5 (maximum specified: 10)

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Site: 101 [FitzroyXAlfred - PM - Design]

New Site Roundabout

Move	Movement Performance - Vehicles													
Mov ID	OD Mov	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop S Rate	Average Speed	
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h	
South	: RoadN	Vame												
2	T1	112	0.0	112	0.0	0.204	3.2	LOS A	1.2	8.2	0.40	0.60	25.1	
3	R2	129	0.0	129	0.0	0.204	7.1	LOS A	1.2	8.2	0.40	0.60	33.5	
Appro	ach	241	0.0	241	0.0	0.204	5.3	LOS A	1.2	8.2	0.40	0.60	30.9	
East:	RoadNa	ame												
4	L2	81	0.0	81	0.0	0.217	4.5	LOS A	1.3	9.3	0.34	0.60	30.2	
6	R2	189	5.0	189	5.0	0.217	9.4	LOS A	1.3	9.3	0.34	0.60	29.7	
Appro	ach	271	3.5	271	3.5	0.217	8.0	LOS A	1.3	9.3	0.34	0.60	29.9	
North	RoadN	lame												
7	L2	409	5.0	409	5.0	0.418	4.8	LOS A	3.1	22.3	0.41	0.50	43.5	
8	T1	134	0.0	134	0.0	0.418	4.9	LOS A	3.1	22.3	0.41	0.50	40.3	
Appro	ach	543	3.8	543	3.8	0.418	4.8	LOS A	3.1	22.3	0.41	0.50	42.9	
All Ve	hicles	1055	2.8	1055	2.8	0.418	5.7	LOS A	3.1	22.3	0.39	0.55	38.0	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.0 % Number of Iterations: 5 (maximum specified: 10)

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V Site: 102 [GlenXAlfred - AM - Ex]

♦ Network: 1 [AM Ex]

New Site Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles Mov OD Demand Flows Arrival Flows Deg. Average Level of 95% Back of Queue Prop. Effective Average													
Mov ID	OD Mov	Demand I Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance	Prop. Queued		Average Speed	
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h	
South	h: RoadN	lame												
2	T1	418	5.0	418	5.0	0.221	0.0	LOS A	0.0	0.0	0.00	0.00	60.0	
Appro	bach	418	5.0	418	5.0	0.221	0.0	NA	0.0	0.0	0.00	0.00	60.0	
North	: RoadN	lame												
8	T1	601	5.0	601	5.0	0.318	0.0	LOS A	0.0	0.0	0.00	0.00	59.9	
Appro	bach	601	5.0	601	5.0	0.318	0.0	NA	0.0	0.0	0.00	0.00	59.9	
West:	RoadN	ame												
10	L2	52	0.0	52	0.0	0.048	7.1	LOS A	0.2	1.3	0.44	0.64	48.2	
Appro	bach	52	0.0	52	0.0	0.048	7.1	LOS A	0.2	1.3	0.44	0.64	48.2	
All Ve	hicles	1071	4.8	1071	4.8	0.318	0.4	NA	0.2	1.3	0.02	0.03	58.9	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.0 % Number of Iterations: 5 (maximum specified: 10)

Number of iterations. 5 (maximum specifica. 10)

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Site: 101 [FitzroyXAlfred - AM - Ex]

New Site Roundabout

Movement Performance - Vehicles Mov OD Demand Flows Arrival Flows Deg. Average Level of 95% Back of Queue Prop. Effective Average													
Mov ID	OD Mov	Demand I Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued		Average Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: Roadl	Vame											
2	T1	94	0.0	94	0.0	0.161	3.9	LOS A	0.9	6.3	0.51	0.64	24.1
3	R2	74	0.0	74	0.0	0.161	7.9	LOS A	0.9	6.3	0.51	0.64	32.9
Appro	ach	167	0.0	167	0.0	0.161	5.7	LOS A	0.9	6.3	0.51	0.64	29.3
East:	RoadNa	ame											
4	L2	76	0.0	76	0.0	0.319	4.5	LOS A	2.1	14.5	0.35	0.61	29.6
6	R2	347	0.0	347	0.0	0.319	9.4	LOS A	2.1	14.5	0.35	0.61	29.0
Appro	ach	423	0.0	423	0.0	0.319	8.5	LOS A	2.1	14.5	0.35	0.61	29.1
North:	RoadN	lame											
7	L2	495	5.0	495	5.0	0.435	4.4	LOS A	3.5	25.4	0.32	0.46	44.3
8	T1	121	0.0	121	0.0	0.435	4.6	LOS A	3.5	25.4	0.32	0.46	41.2
Appro	ach	616	4.0	616	4.0	0.435	4.4	LOS A	3.5	25.4	0.32	0.46	43.8
All Ve	hicles	1206	2.1	1206	2.1	0.435	6.0	LOS A	3.5	25.4	0.36	0.54	37.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.0 % Number of Iterations: 5 (maximum specified: 10)

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V Site: 102 [GlenXAlfred - PM - Ex]

♦ Network: 1 [PM Ex]

ed

60.0

60.0

59.9

59.9

48.5

48.5

58.3

New Site Giveway / Yield (Two-Way)

Mov	ement I	Performa	nce - \	/ehicle	s								
Mov	OD	Demand	Flows	Arriva		Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective /	Aver
ID	Mov	Total	ΗV	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Spe
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	k
South	n: RoadN	lame											
2	T1	305	5.0	305	5.0	0.162	0.0	LOS A	0.0	0.0	0.00	0.00	6
Appro	oach	305	5.0	305	5.0	0.162	0.0	NA	0.0	0.0	0.00	0.00	6
North	n: RoadN	lame											
8	T1	476	5.0	476	5.0	0.252	0.0	LOS A	0.0	0.0	0.00	0.00	Ę
Appro	oach	476	5.0	476	5.0	0.252	0.0	NA	0.0	0.0	0.00	0.00	Ę
West	: RoadN	ame											
10	L2	65	0.0	65	0.0	0.053	6.6	LOS A	0.2	1.4	0.37	0.60	2
Appro	oach	65	0.0	65	0.0	0.053	6.6	LOS A	0.2	1.4	0.37	0.60	2
All Ve	ehicles	846	4.6	846	4.6	0.252	0.5	NA	0.2	1.4	0.03	0.05	Ę

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.0 % Number of Iterations: 5 (maximum specified: 10)

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Site: 101 [FitzroyXAlfred - PM - Ex]

New Site Roundabout

Move	Movement Performance - Vehicles Mov OD Demand Flows Arrival Flows Deg. Average Level of 95% Back of Queue Prop. Effective Average													
Mov ID	OD Mov	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued		Average Speed	
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h	
South	: RoadN	Vame												
2	T1	112	0.0	112	0.0	0.182	3.2	LOS A	1.0	7.1	0.39	0.58	25.4	
3	R2	103	0.0	103	0.0	0.182	7.1	LOS A	1.0	7.1	0.39	0.58	33.8	
Appro	ach	215	0.0	215	0.0	0.182	5.0	LOS A	1.0	7.1	0.39	0.58	30.8	
East:	RoadNa	ame												
4	L2	58	0.0	58	0.0	0.193	4.4	LOS A	1.1	8.0	0.30	0.59	30.2	
6	R2	189	5.0	189	5.0	0.193	9.3	LOS A	1.1	8.0	0.30	0.59	29.6	
Appro	ach	247	3.8	247	3.8	0.193	8.2	LOS A	1.1	8.0	0.30	0.59	29.8	
North	RoadN	lame												
7	L2	409	5.0	409	5.0	0.387	4.6	LOS A	2.8	20.1	0.35	0.48	44.0	
8	T1	111	0.0	111	0.0	0.387	4.7	LOS A	2.8	20.1	0.35	0.48	40.9	
Appro	ach	520	3.9	520	3.9	0.387	4.6	LOS A	2.8	20.1	0.35	0.48	43.5	
All Ve	hicles	982	3.0	982	3.0	0.387	5.6	LOS A	2.8	20.1	0.35	0.53	38.5	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.0 % Number of Iterations: 5 (maximum specified: 10)

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